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GLASTONBURY, CONNECTICUT 06033
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**HYDROLOGY AND HYDRAULICS &
NITROGEN LOADING
ENGINEERING REPORT**

**GENERAL LANDSCAPING, LLC
121 KREIGER LAND, GLASTONBURY, CT**

January 13, 2022

Prepared By:

**Mark W. Friend, PE
Soil Scientist, LEED AP**

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Summary

In general, the project is a proposed 15 space paved parking lot and landscape materials storage area on 121 Kreiger Lane. It is intended to be an extension of the existing operation General Landscaping, LLC currently operates at 100 and 116 Kreiger Lane directly across the street. The paved area is proposed to be drained via both inlet and pipe system as well as sheet flow to the south and into a stormwater management basin (SWMB).

This basin will treat the water quality volume and dissipate it into a coarse aggregate layer of bedding material. Underlying soil conditions, as evidenced by a test deep pit, indicate coarse grained soils as well as a deep groundwater table. This will allow storage and infiltration of the water quality volume as well as reduce increases in peak flows from the 2 to 100 years storms.

Methodology

Peak flows were developed using the Rational Method. These were used to analyze the capacity of the existing town drainage system serving Kreiger Lane to ensure adequacy for a 10 year return frequency storm. This system was designed, with the original subdivision, in anticipation of full development of all the industrial lots fronting on the street. The Point Precipitation Frequency Estimates were taken from NOAA Atlas 14, Volume 10, Version 3 as developed for the Town of Glastonbury. The drainage areas to each catch basin are shown on the Drainage Area Map in the appendices. The Water Quality Volume was computed using the equation in the 2004 Connecticut Stormwater Quality Manual.

Required Water Quality Volume

$$WQV = \frac{(1'')(R)(A)}{12} \quad \text{Where}$$

$$R = 0.05 + 0.009(I) \quad 0.05 + 0.009(70) = 0.680$$

$$I = \% \text{ Impervious} = \frac{27,878 \text{ S.F.}}{40,073 \text{ S.F.}} = 70\%$$

$$A = \text{area of the catchment} = 0.92 \text{ AC}$$

$$WQV = \frac{(1'')(0.680)(0.92)}{12} = 0.052 \text{ AC-FT} = \underline{\underline{2,271 \text{ CF}}}$$

$$\text{WQV provided} - 4,671 \text{ CF} > 2,271 \text{ CF}$$

Conclusions

- The existing Town drainage system will adequately convey a 10 year return frequency storm without surcharge in accordance with the Town of Glastonbury Engineering Department requirements. This is consistent with the original design of the Kreiger Lane drainage system which anticipated full development of the industrial lots
- The proposed SWMB will provide a storage capacity in excess of 200% of the Water Quality Volume (WQV) which is 2,271 CF.
- No adverse impacts from development will be created for downstream areas.
- The Water Quality Volumes from the project will be collected and infiltrated within the SWMB proposed.
- The proposed Stormwater Management Measures exceed the goals of the 2004 Connecticut Stormwater Quality Manual.

GENERAL LANDSCAPING, LLC
121 KREIGER LANE, GLASTONBURY

APPENDIX A
HYDRAULIC GRADE LINE (HGL) CALCULATIONS

Storm Sewer Summary Report

Line No.	Line ID	Flow rate (cfs)	Line size (in)	Line length (ft)	Invert EL Dn (ft)	Invert EL Up (ft)	Line slope (%)	HGL down (ft)	HGL up (ft)	Minor loss (ft)	HGL Junct (ft)	Dns line No.
1	FLEND TO 19+25	39.70	42 c	48.0	80.96	81.21	0.521	83.00	83.22	0.38	83.59	End
2	19+25 TO 17+50	37.50	30 c	172.0	82.05	90.56	4.948	84.10	92.61	n/a	92.61 j	1
3	17+50 TO 16+33	35.80	30 c	110.0	90.56	92.27	1.555	92.96	94.27	n/a	94.27 j	2
Project File: New.stm							Number of lines: 3			Run Date: 01-13-2022		
NOTES: c = cir; e = ellip; b = box; Known Qs only ; j - Line contains hyd. jump.												

Hydraulic Grade Line Computations

Line	Size (in)	Q (cfs)	Downstream								Len (ft)	Upstream								Check		JL coeff (K)	Minor loss (ft)
			Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)		Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)	Ave Sf (%)	Enrgy loss (ft)		
1	42	39.70	80.96	83.00	2.04	5.82	6.82	0.72	83.72	0.502	48.0	81.21	83.22	2.01	5.71	6.95	0.75	83.97	0.528	0.515	0.247	0.50	0.38
2	30	37.50	82.05	84.10	2.05*	4.30	8.72	1.18	85.28	1.115	172	90.56	92.61 j	2.05**	4.30	8.72	1.18	93.79	1.115	1.115	n/a	0.90	n/a
3	30	35.80	90.56	92.96	2.40	4.84	7.39	0.85	93.81	0.884	110	92.27	94.27 j	2.00**	4.21	8.51	1.12	95.39	1.062	0.973	n/a	1.00	n/a

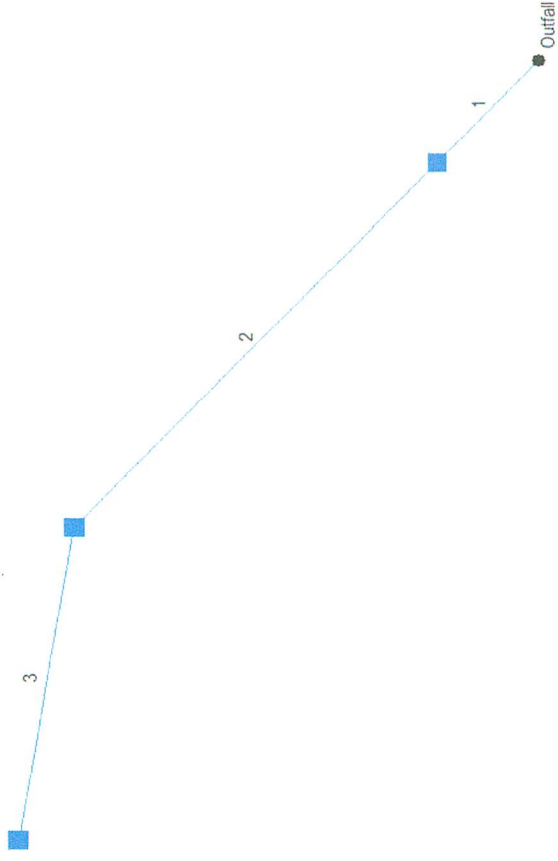
Project File: New.stm

Number of lines: 3

Run Date: 01-13-2022

Notes: * Critical depth assumed.; ** Critical depth.; j-Line contains hyd. jump.

Hydraflow Plan View



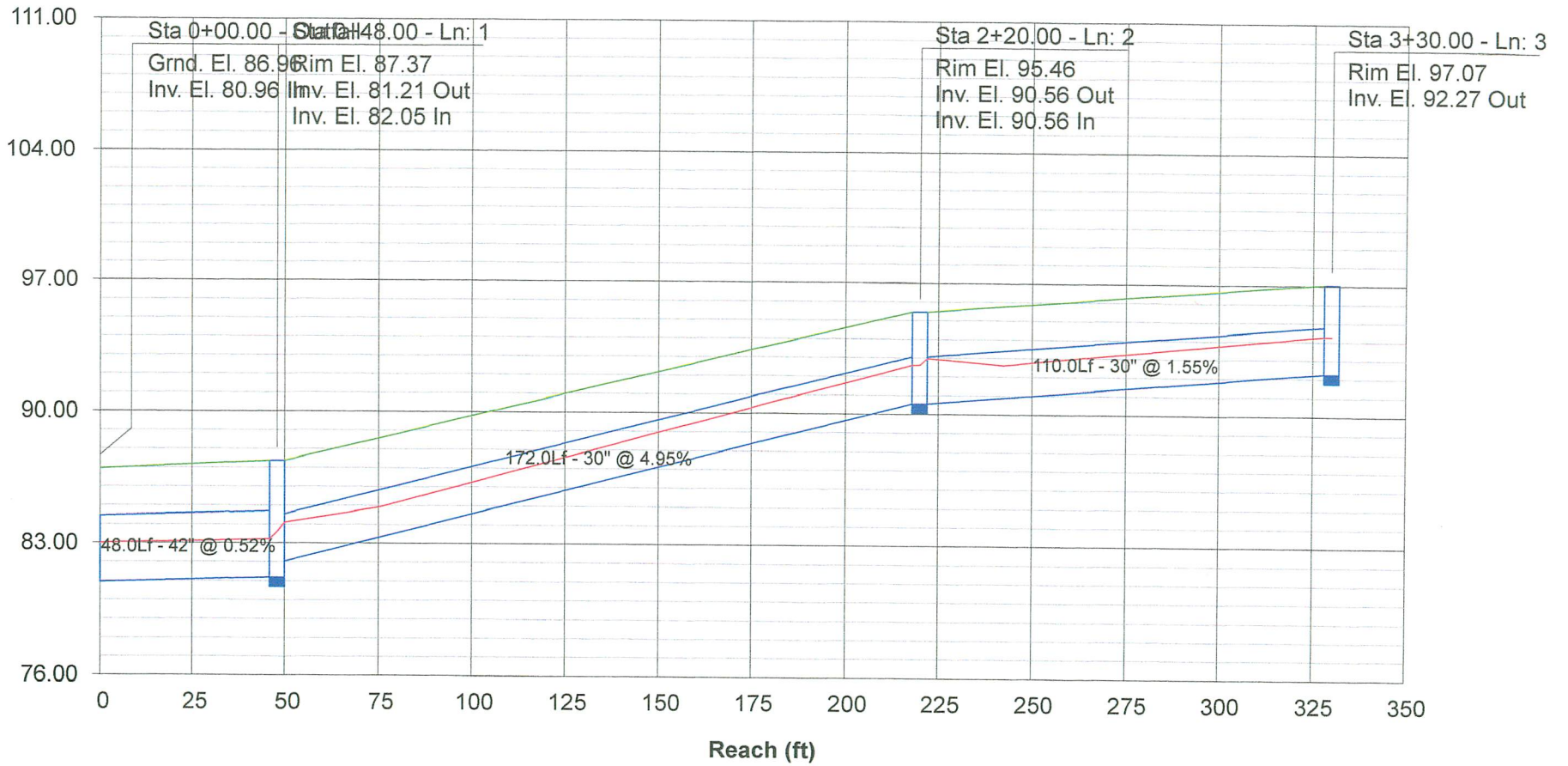
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No. Lines: 3

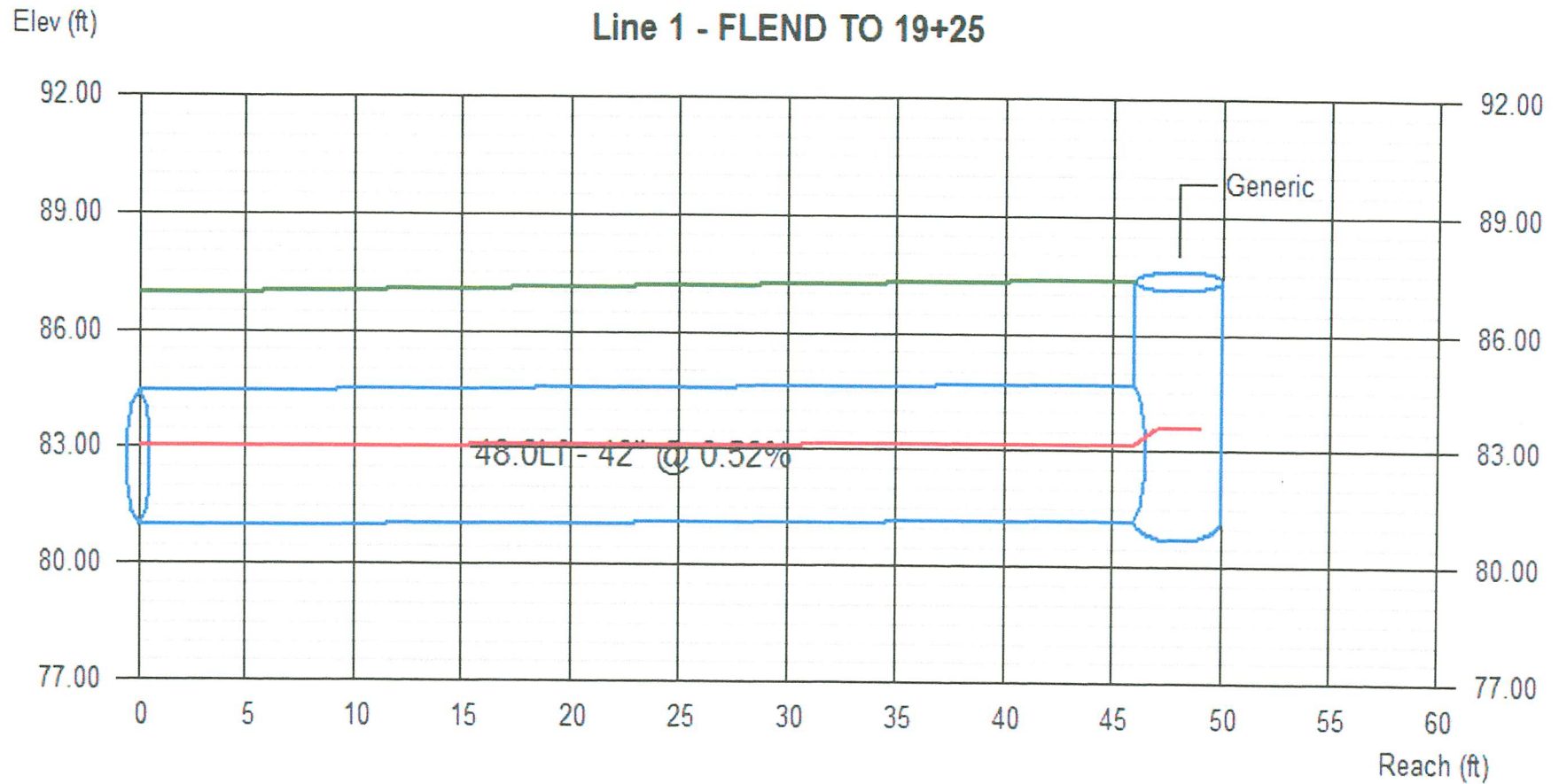
01-13-2022

Storm Sewer Profile

Elev. (ft)



Line Profile (Line 1) - FLEND TO 19+25



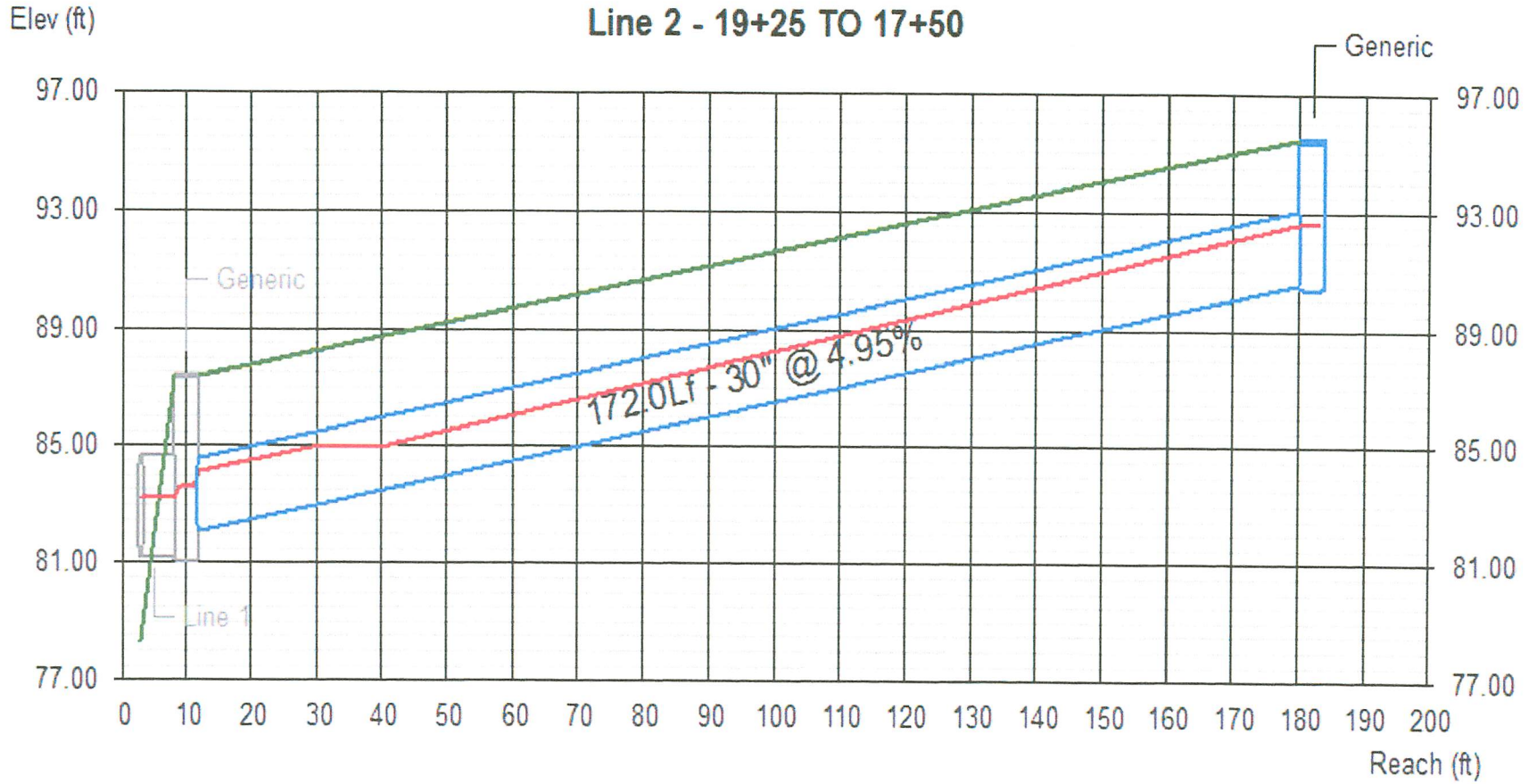
Line #	Q (cfs)	Invert Elevation		Depth of Flow			Hydraulic Grade Line			Velocity		Cover	
		Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	Hw (ft)	Dn (ft)	Up (ft)	Jnct (ft)	Dn (ft/s)	Up (ft/s)	Dn (ft)	Up (ft)
1	39.70	80.96	81.21	2.04	2.01	2.38	83.00	83.22	83.59	6.82	6.95	2.50	2.66

Project File:

No. Lines: 3

Run Date: 01-13-2022

Line Profile (Line 2) - 19+25 TO 17+50



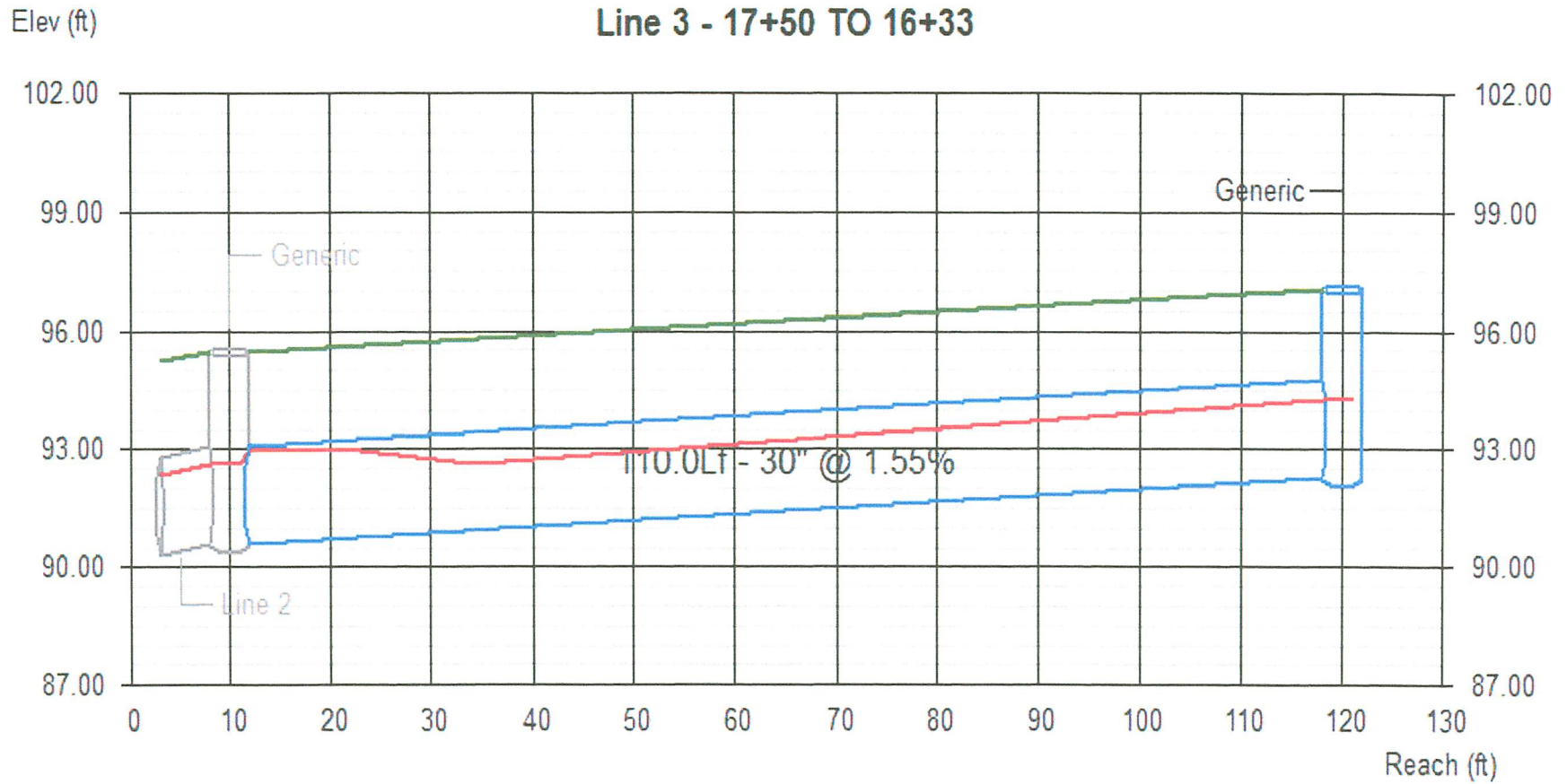
Line #	Q (cfs)	Invert Elevation		Depth of Flow			Hydraulic Grade Line			Velocity		Cover	
		Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	Hw (ft)	Dn (ft)	Up (ft)	Jnct (ft)	Dn (ft/s)	Up (ft/s)	Dn (ft)	Up (ft)
2	37.50	82.05	90.56	2.05	2.05	2.05	84.10	92.61 j	92.61	8.72	8.72	2.82	2.40

Project File:

No. Lines: 3

Run Date: 01-13-2022

Line Profile (Line 3) - 17+50 TO 16+33



Line #	Q (cfs)	Invert Elevation		Depth of Flow			Hydraulic Grade Line			Velocity		Cover	
		Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	Hw (ft)	Dn (ft)	Up (ft)	Jnct (ft)	Dn (ft/s)	Up (ft/s)	Dn (ft)	Up (ft)
3	35.80	90.56	92.27	2.40	2.00	2.00	92.96	94.27 j	94.27	7.39	8.51	2.40	2.30

Project File: _____ No. Lines: 3 Run Date: 01-13-2022

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APPENDIX B
DRAINAGE AREAS &
STORM SEWER DESIGN SHEET

RUNOFF TO CB STA 16+33

AREA - 9.15 AC IMPERVIOUS 4.87 AC PERVIOUS

$$\text{WEIGHTED } C_{\text{IMP}} = \frac{(9.15)(0.9) + (4.87)(0.3)}{14.02 \text{ AC}}$$
$$= 0.69$$

$T_c = 20 \text{ MIN}$ (FROM ORIGINAL DRAINAGE REPORT)

$I = 3.7$ (NOM NWS 1A, VOL 10, V. 3)

$$AI = (14.02)(0.69) = 9.67$$

RUNOFF TO CB STA 17+50

AREA - 9.60 AC IMPERVIOUS 5.10 AC PERVIOUS

$$\text{WEIGHTED } C_{\text{IMP}} = \frac{(9.60)(0.9) + (5.10)(0.3)}{14.70 \text{ AC}}$$
$$= 0.69$$

$T_c = 20 \text{ MIN}$

$I = 3.7$

$$AI = (14.70)(0.69) = 10.14$$

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JOB 131-20 GENERAL LANDSCAPE

SHEET NO. _____ OF _____

CALCULATED BY MWF DATE 1-12-22

CHECKED BY _____ DATE _____

SCALE _____

RUNOFF TO CB STA 19+25

AREA - 10.35 AC IMPERVIOUS 5.89 AC PERVIOUS

$$\begin{aligned} \text{WEIGHTED } C_{\text{IMP}} &= \frac{(10.35)(0.9) + (5.89)(0.3)}{16.24 \text{ AC}} \\ &= 0.68 \end{aligned}$$

$$T_c = 21 \text{ MIN}$$

$$I = 3.6$$

$$A_1 = (16.24)(0.68) = 11.04$$

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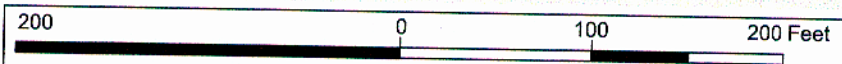
APPENDIX C
DRAINAGE AREAS MAP



Legend

- Property Line
- Address Points
- Sanitary Pump Stations
- Sanitary Manhole
 - Active, Gravity
 - Active, Pressure
 - Capped, Gravity
- Sanitary Gravity Main
 - Active, 6" - 8"
 - Active, 10" - 18"
 - Active, 24" - 36"
 - Capped, 8"
- Sanitary Force Main
 - 2" - 4"
 - 10" - 18"
- Storm Inlet Structures
 - Catch Basin
 - Flared End
 - Headwall
 - Overflow
 - Pipe End
 - Yard Drain
- Storm Outlet Structures
 - End Wall
 - Flared End
 - Pipe End
- Storm Manhole
- Storm Gravity Lines
 - Culvert, 42" - Larger
 - Culvert, 15" - 36"
 - Collector, 64" - 84"
 - Collector, 30" - 46"
 - Collector, 18" - 27"
 - Collector, 1" - 15"
- Storm Open Channel
- Storm Lines - (Abandoned)
- Storm Detention Ponds
- Dams
- Glastonbury Fire Hydrants
- Manchester Fire Hydrants
- MDC Fire Hydrants
- Sewer Easement
- Drainage Easement
- Street Centerlines
 - State Highway
 - Secondary Roads
- Utility Poles, Towers and Lights
 - Utility poles without street light
 - Utility pole with street light
 - Light Pole
 - Transmission or Cellphone Tower
 - Electric Box for traffic signals
- Buildings
 - Building
 - Out building
 - Area under construction
 - Building foundation
 - Mobile home, trailer
 - Ruined building or structure
- Ancillary Structures
 - Solar Panels on ground
 - Smokestack
 - Water storage tank
 - Dam Structure
 - Bus Shelter
 - Deck
 - Flight of Stairs
 - Fuel storage tank
 - Generator
 - Miscellaneous Structure
 - Porch
 - Swimming pool above ground
 - Swimming pool inground

1-13-20
GENERAL LANDSCAPE
131-20
DRAINAGE
AREAS



4000

1:1,200

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APPENDIX D
STORMWATER MANAGEMENT BASIN
VOLUME CALCULATIONS

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APPENDIX E
NITROGEN LOADING CALCULATIONS

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January 14, 2022

131-20 NITROGEN LOADING CALCULATIONS

GENERAL LANDSCAPING, LLC – 121 KREIGER LANE, GLASTONBURY

PARCEL AREA: 40,073 SF = 0.920 AC.

IMPERVIOUS SURFACE

PAVEMENT = 27,878 SF

LAWN AREA = 9,695 SF

NATURAL AREA = 2,500 SF

CONNECTICUT RAINFALL = 44 IN./YR

LOADING FACTORS:

CAPE COD TECH BULLETIN 91-001

DRIVES – 1.5 mg/L

LAWNS – 3 LBS / 1000 SF @ 40 % LEACHING

NATURAL AREA INFILTRATION – 40%

ESTIMATED LOADING

PAVED AREA:

$$27,878 \text{ SF} \times 44 \text{ IN/YR} \times 1 \text{ FT}/12 \text{ IN} \times 1 \text{ YR}/365 \text{ D} \times 28.32 \text{ L/CF} \\ = 7,931 \text{ L/D} \times 1.5 \text{ mg/L} = 11,897 \text{ mg/D}$$

LAWN:

$$9,695 \text{ SF} \times 3 \text{ LBS}/1000 \text{ SF/YR} \times 1 \text{ YR}/365 \text{ D} \times 454,000 \text{ mg/LB} \times 0.4 \\ = 14,471 \text{ mg/D}$$

NATURAL AREA:

$$2,500 \text{ SF} \times 44 \text{ IN/YR} \times 1 \text{ FT}/12 \text{ IN} \times 1 \text{ YR}/365 \text{ D} \times 28.32 \text{ L/CF} \times 0.4 \\ = 284 \text{ L/D}$$

SUMMARY:

CONCENTRATION		
PAVED AREA	+	LAWNS
11,897 mg/D		14,471 mg/D
<hr/>		
284 L/D		7,931 L/D
NATURAL AREA	+	PAVED AREA
<hr/>		
$= (26,368 \text{ mg/D}) / (8,215 \text{ L/D}) = 3.21 \text{ mg/L}$		

THIS SITE IS LOCATED IN A GW-1 ZONE

NITROGEN LOADING OF 3.21 mg/L IS WITHIN THE ACCEPTABLE RANGE OF SECTION 20.13.1 OF THE GROUNDWATER PROTECTION REGULATIONS.

GENERAL LANDSCAPING, LLC
121 KREIGER LANE, GLASTONBURY

APPENDIX F
ORIGINAL DRAINAGE REPORT
OAK STREET INDUSTRIAL SUBDIVISION
FEBRUARY 20, 1986

DRAINAGE CALCULATIONS
FOR
OAK STREET INDUSTRIAL SUBDIVISION

FEBRUARY 20, 1986

MEGSON & HEAGLE

CIVIL ENGINEERS & LAND SURVEYORS

GLASTONBURY, CONNECTICUT

Attached are the drainage calculations for the Oak Street Subdivision. The storm drainage system is designed based on the assumption that 70% of each lot will be either pavement or building coverage and 30% will be grassed. The assumption is also made that each lot will have a storm drainage system for its parking area and the design of the proposed system takes that into consideration.

All catchbasins from station 9+35 to station 19+75 will discharge into Hubbard Brook. The discharge will be 33.2 CFS at a velocity of 4.5 FPS for a 10 year frequency storm.

All catchbasins from station 6+40 to 1+05 will connect to the system proposed for the Oak Street reconstruction. The discharge into this system will be 21.2 CFS at a time of concentration of 25 minutes for a ten year frequency storm. This connection has been coordinated with the design of the Oak Street reconstruction with the Town engineering staff.

MEGSON & HEAGLE

CIVIL ENGINEERS & LAND SURVEYORS
81 RANKIN ROAD
GLASTONBURY, CONNECTICUT 06033

MGM

Project No. 99-84
Sheet No. 1 of 2 Sheets
Date 1-22-86
Designed By MWF
Checked By _____

RUNOFF FROM PARKING TO CB STA 9+35 LT

.65 AC PAVED .25 AC GRASSED
 $AI = (.65 \times .9) + (.25 \times .3) = \underline{.66}$

RUNOFF FROM PARKING TO CB STA 9+35 RT

1.30 AC PAVED .50 AC GRASSED
 $AI = (1.30 \times .9) + (.50 \times .3) = \underline{1.32}$

RUNOFF FROM PARKING TO CB 11+65 LT

2.05 AC PAVED .65 AC GRASSED
 $AI = (2.05 \times .9) + (.65 \times .3) = \underline{2.04}$

RUNOFF FROM PARKING TO CB 11+65 RT

.65 AC PAVED .25 AC GRASSED
 $AI = \underline{.66}$

RUNOFF FROM PARKING TO CB 14+85 LT

1.65 AC PAVED .25 AC GRASSED
 $AI = .66$

RUNOFF FROM PARKING TO CB 14+85 RT

1.30 AC PAVED .25 AC GRASSED
 $AI = \underline{1.32}$

RUNOFF FROM PARKING TO PIPE BETWEEN 14+85 RT & 17+50

1.10 AC PAVED 2.0 AC GRASSED
 $AI = (1.1 \times .9) + (2.0 \times .3) = \underline{1.59}$
 $T_c = 20 \text{ MIN}$

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81 RANKIN ROAD
GLASTONBURY, CONNECTICUT 06033

MGM
Project No. 99-89
Sheet No. 2 of 2 Sheets
Date 1-23-86
Designed By MWF
Checked By _____

RUNOFF FROM PARKING TO CB STA 6+40 LT & RT

.65 AC PAVED

.25 AC GRASSED

$$AI = \underline{.66}$$

RUNOFF FROM PARKING TO CB STA 4+80 RT

1.70 AC PAVED

.70 AC PAV

$$AI = (1.70)(.9) + (.70)(.3) = \underline{1.74}$$

RUNOFF FROM PARKING TO CB 4+80 LT

$$AI = .66$$

Storm Sewer System Design 10 YR STORM

Line Segment	Time to Inlet	Time in Pipe	Accumulated Time	AI Entering Catch Basin	Sum of AI in System	Rainfall Intensity	Q in System	Pipe Size	Length of Pipe	Slope	Average Velocity	Full Capacity	Headwater (FT)	n
9+35 LT	13	-	13	.09	.75	4.3	3.2	15"	24'	1.0%	4.8	5.7	-	-
9+35 RT	13	37	13	.09	2.16	4.3	9.3	21"	230'	1.0%	6.1	14	-	-
11+65 RT	12	-	12	.08	2.12	4.4	9.3	21"	29'	1.0%	6.1	14	-	-
11+65 LT	12	50	14	.08	5.02	4.1	20.6	27"	320'	0.7%	6.5	23	2.5	-
14+85 RT	14	-	14	.12	.78	4.1	3.2	15"	24'	1.0%	4.8	5.7	-	-
14+85 LT	14	33	20	.12	8.83	3.4	30.0	30"	265'	1.0%	8.0	36	3.1	-
17+50 RT	17	-	20	.56	9.39	3.4	32.0	30"	175'	5.0%	-	80	3.1	-
17+50 LT	21	-	21	.66	10.05	3.3	33.2	42"	50'	.02%	4.5	40	2.6	-
6+40 RT	10	-	10	0	.66	4.7	3.1	15"	24'	1.0%	4.8	5.7	-	-
6+40 LT	10	20	10	0	1.32	4.7	6.2	15"	160'	3.0%	8.2	9.9	1.9	-
4+80 RT	10	-	10	.07	3.13	4.7	14.7	21"	24'	1.5%	8.0	17	2.6	-
4+80 LT	10	16	10	.07	3.86	4.7	18.1	21"	180'	3.0%	11	24	3.5	-
3+00 RT	10	17	11	.09	3.95	4.5	18.1	21"	190'	3.0%	11	24	3.5	-
3+00 LT	13	-	13	.16	1.6	4.3	6.9	15"	32'	1.0%	3	5.7	-	-
1+05 RT	25	-	25	2.74	6.85	3.1	21.24	21"	35'	3.0%	11	24	3.5	-
1+05 LT														
EXIST BASIN														

Project 94-84
 Town GLAST.
 Designed by MWF
 Checked by
 Sheet No. 1-22-86

Gutter Flow Analysis 10 YR STORM

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Inlet Number	Time to Inlet	Area in Acres	Runoff Coefficient	AI	Sum of AI	Total AI	Rainfall Intensity	Q to Inlet	Grade of Gutter %	Cross Slope of % Shoulder	Depth of Flow at Gutter	Width of Flow	Q Bypassing Inlet	AI Bypassing Inlet	AI Entering Catch Basin
9+352T	13	.06	.3	.06	.09	.09	4.3	.39	1	3	-	-	-	-	.09
11+65LT	12	.07	.3	.06	.08	.08	4.4	.40	1	3	-	-	-	-	.08
14+85LT	14	.10	.3	.09	.12	.12	4.1	.49	1	3	-	-	-	-	.12
17+50	17	.33	.3	.26	.56	.56	3.8	2.13	0	0	-	-	-	-	.56
9+35RT	13	.11	.3	.06	.09	.09	4.3	.39	1	3	-	-	-	-	.09
11+65RT	12	.07	.3	.06	.08	.08	4.4	.40	1	3	-	-	-	-	.08
14+85RT	14	.10	.3	.09	.12	.12	4.1	.49	1	3	-	-	-	-	.12
4+80LT	10	.04	.3	.04	.07	.07	4.7	.33	1	3	-	-	-	-	.07
3+00LT	10	.05	.3	.05	.09	.09	4.7	.42	1	3	-	-	-	-	.09
1+05LT	25	.13	.3	.27	2.74	2.74	3.1	0.49	1	3	-	-	-	-	2.74
4+80RT	10	.04	.3	.04	.07	.07	4.7	.33	1	3	-	-	-	-	.07
1+10RT	13	.21	.3	.18	.16	.16	4.3	.69							.16

Town GLAST.

Checked by

Project 9A-84
OAK STREET

Designed by
MLF

Sheet No.

GIVEN: $D_o = 3.5'$
 $T_w = .72$

$Q = 33.2$ CFS $V = 4.5$ FPS

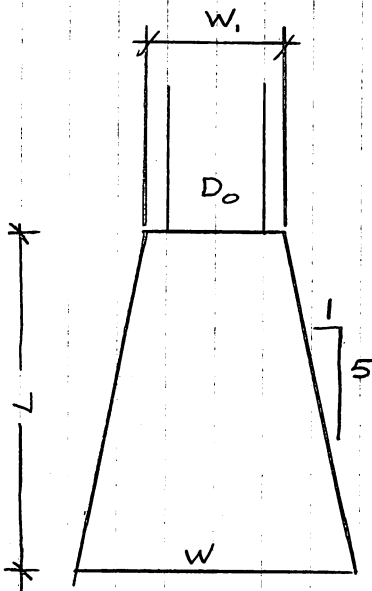
$$L_a = \frac{1.7Q}{D_o^{3/2}} + 8D_o$$

$$L_a = \frac{(1.7)(33.2)}{(3.5)^{3/2}} + 8(3.5)$$

$$= 36.6 \text{ FT} \approx \underline{\underline{40 \text{ FT}}}$$

$$W = 3D_o + 0.4L_a$$

$$W = (3)(3.5) + (0.4)(40) = \underline{\underline{26.5 \text{ FT}}}$$



$$W_1 = 3D_o = \underline{\underline{10.5 \text{ FT}}}$$

$$D_{50} = \frac{0.02}{T_w} \left(\frac{Q}{D_o} \right)^{4/3} = \left(\frac{0.02}{.72} \right) \left(\frac{33.2}{3.5} \right)^{4/3}$$

$$= .56 \text{ FT} = 7 \text{ INCHES}$$

USE INTERMEDIATE RIP RAP

SO

$W_1 = 10.5 \text{ FT}$
 $L = 40.0 \text{ FT}$
 $W = 26.5 \text{ FT}$

GENERAL LANDSCAPING, LLC
121 KREIGER LANE, GLASTONBURY

APPENDIX G
OAK STREET INDUSTRIAL SUBDIVISION
ROADWAY RECORD DRAWINGS

PROP WATER NOTES

- 1) GATE VALVE OPERATIONS FOR THIS PROPOSED PROJECT SHALL OPEN LEFT.
- 2) ALL GATE VALVES AND BLOW-OFFS SHALL BE FURNISHED WITH GATE BOXES.
- 3) ALL FITTINGS SHALL BE MECHANICAL JOINT TYPE, UNLESS OTHERWISE NOTED.
- 4) MINIMUM DEPTH OF COVER OVER WATER MAIN SHALL BE 4.5 FEET.
- 5) ALL MATERIALS & WORKMANSHIP SHALL CONFORM WITH THE WATER SERVICE MANUAL OF THE M.D.C. REVISED JAN. 1985.

NOTE:
GAS MAIN SHALL BE LOCATED WITHIN TRAVELED WAY, 5' ± SOUTH OF NORTH CURB PER C.N.G.

LENGTH OF RESTRAINT:
 $\frac{295.0'}{210.5' \text{ HEAD}} = \frac{210.5' \times 49.0089}{514.16} = 32.83'$

OAK STREET INDUSTRIAL SUBDIVISION PLANNED INDUSTRIAL SUBDIVISION NAME ZONE

M G M ASSOCIATES
SUBDIVIDER

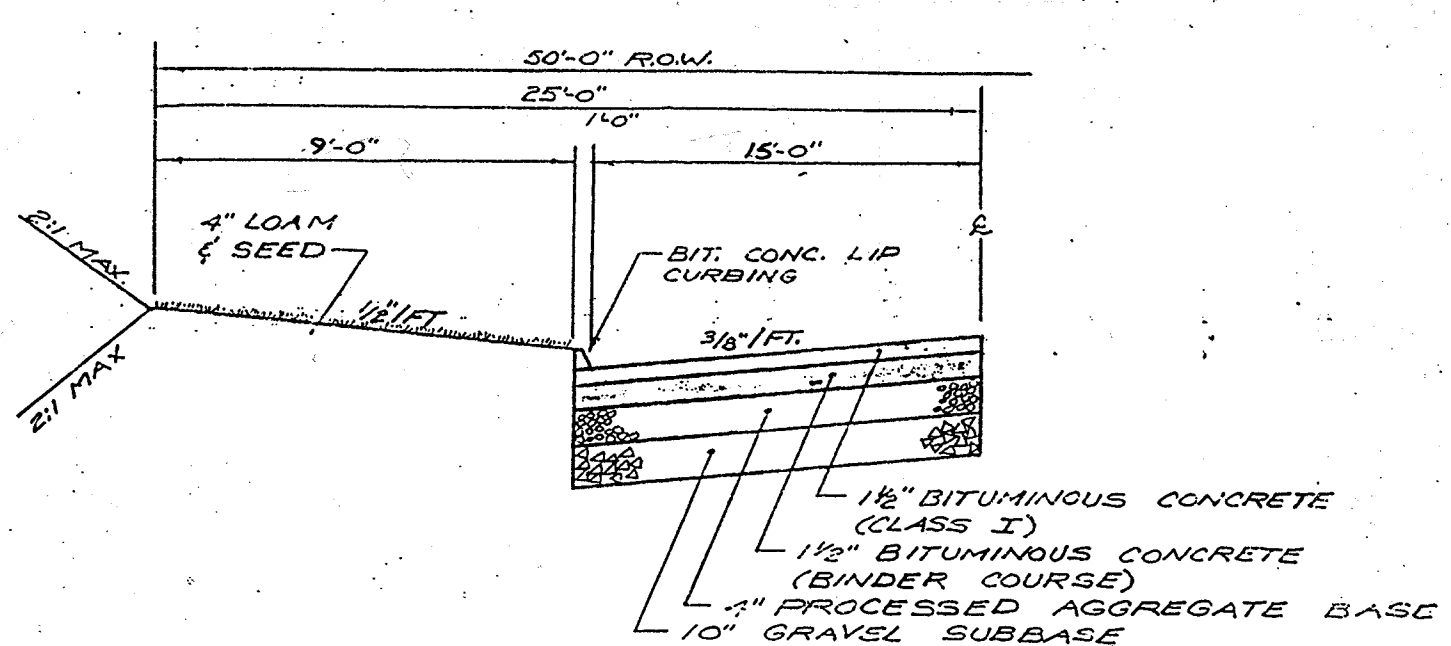
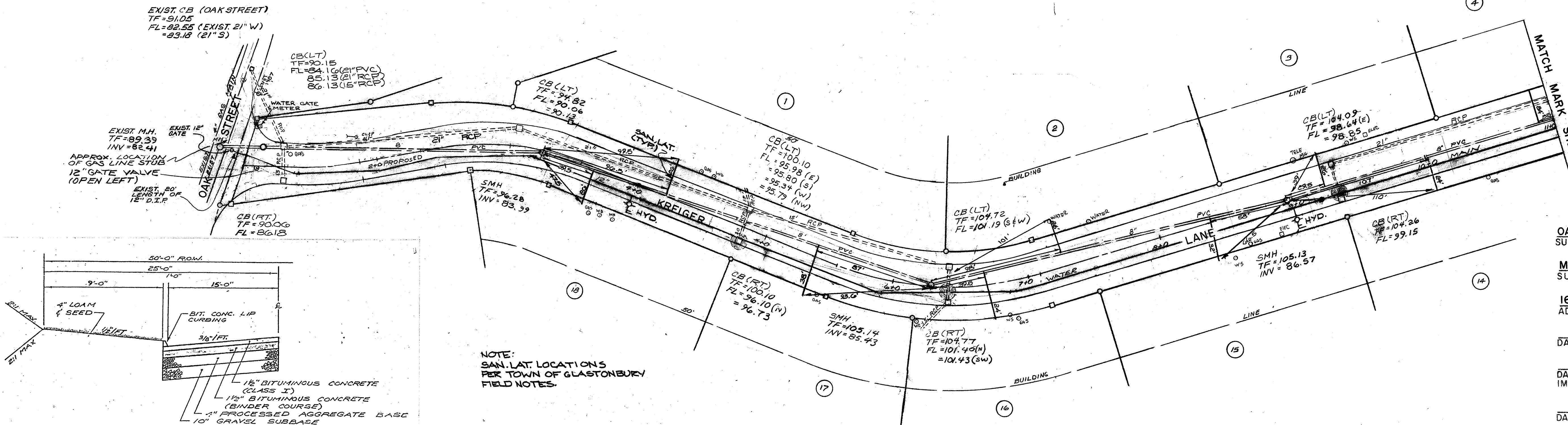
160 OAK STREET GLASTONBURY, CONN. 06033
ADDRESS

DATE APPLICATION RECEIVED COMMUNITY DEVELOPMENT DIRECTOR

DATE PUBLIC IMPROVEMENTS APPROVED DIRECTOR OF PUBLIC WORKS

DATE SUBDIVISION APPROVED PLAN AND ZONING COMMISSION CHAIRMAN

DATE SUBDIVISION EXPIRES PLAN AND ZONING COMMISSION SECRETARY FILE NO.



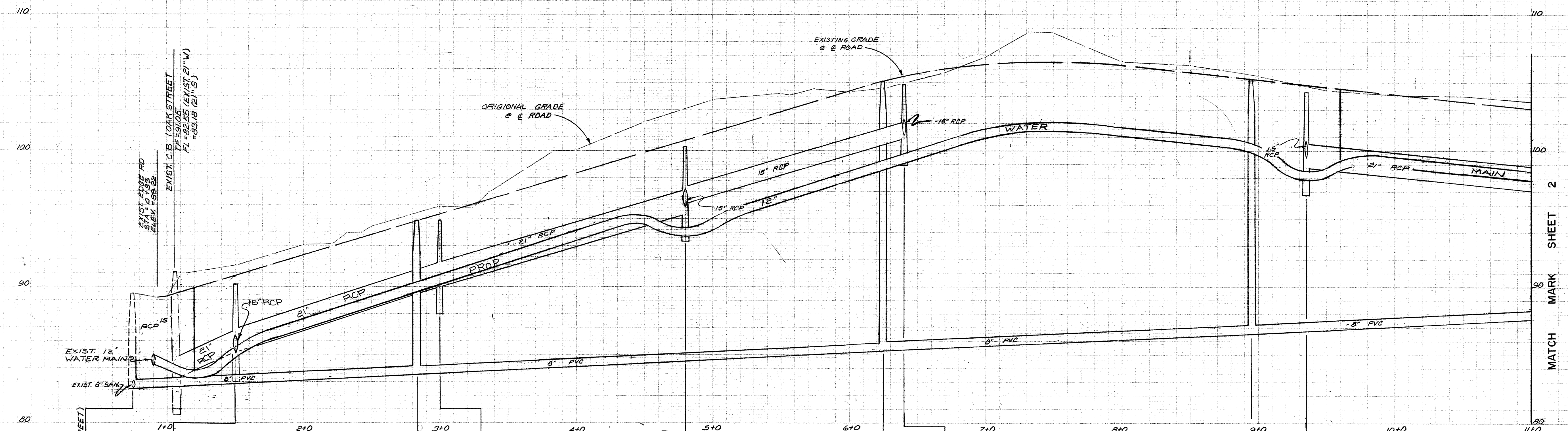
NOTE:
SAN. LAT. LOCATIONS PER TOWN OF GLASTONBURY FIELD NOTES.

PROPOSED KREIGER LANE
HALF SECTION
(NOT TO SCALE)

Bill G. Jan...
APPROVED - TOWN ENGINEER - WATER ONLY - HYDRANT LOCATIONS PER FIRE MARSHAL

Robert Mann...
APPROVED - DEVELOPER

SCALE:
PLAN: 1" = 40'
PROFILE: 1" = 40' HORIZ.
1" = 4' VERT.



"RECORD DRAWING"
ROAD DRAINAGE
SANITARY & WATER
PLAN & PROFILE
KREIGER LANE
OAK STREET
INDUSTRIAL SUBDIVISION
PREPARED FOR
M G M ASSOCIATES
GLASTONBURY, CONN.

REV 10-26-86 RECORD DRAWING
REV 3-3-87 STORM SEWER
REV 2-3-87 MDS COMMENTS

MEGSON & HEAGLE
CIVIL ENGINEERS & LAND SURVEYORS
GLASTONBURY, CONN.

DATE 12-18-86 SCALE: SHOWN MAP NO. 94-84-ID
SHEET 1 OF 2 SHEETS

CERTIFIED SUBSTANTIALLY CORRECT

EXIST. M.H. (OAK STREET) TF=89.39 INV=82.41
 CB(LT) TF=90.15 FL=84.16(21" PVC) 85.13(21" RCP) 86.13(15" RCP)
 CB(LT) TF=94.82 FL=90.06 INV=90.12
 SMH TF=96.28 INV=83.39
 CB(LT) TF=100.10 FL=95.98 (E) 95.80 (S) 95.34 (W) 95.79 (NW)
 CB(LT) TF=104.72 FL=101.19 (S & W)
 CB(LT) TF=104.09 FL=98.64 (E) 98.85 (S)
 SMH TF=105.13 INV=86.57
 CB(RT) TF=104.26 FL=99.15
 CB(RT) TF=104.77 FL=101.40 (N) 101.43 (SW)
 SMH TF=105.14 INV=85.43
 CB(RT) TF=100.10 FL=96.10 (W) 96.73
 SMH TF=105.14 INV=85.43
 CB(RT) TF=104.77 FL=101.40 (N) 101.43 (SW)
 SMH TF=105.13 INV=86.57
 CB(RT) TF=104.26 FL=99.15
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 SMH TF=105.13 INV=86.57
 CB(RT) TF=104.26 FL=99.15

M G M ASSOCIATES
 SUBDIVIDER

160 OAK STREET GLASTONBURY, CONN. 06033
 ADDRESS

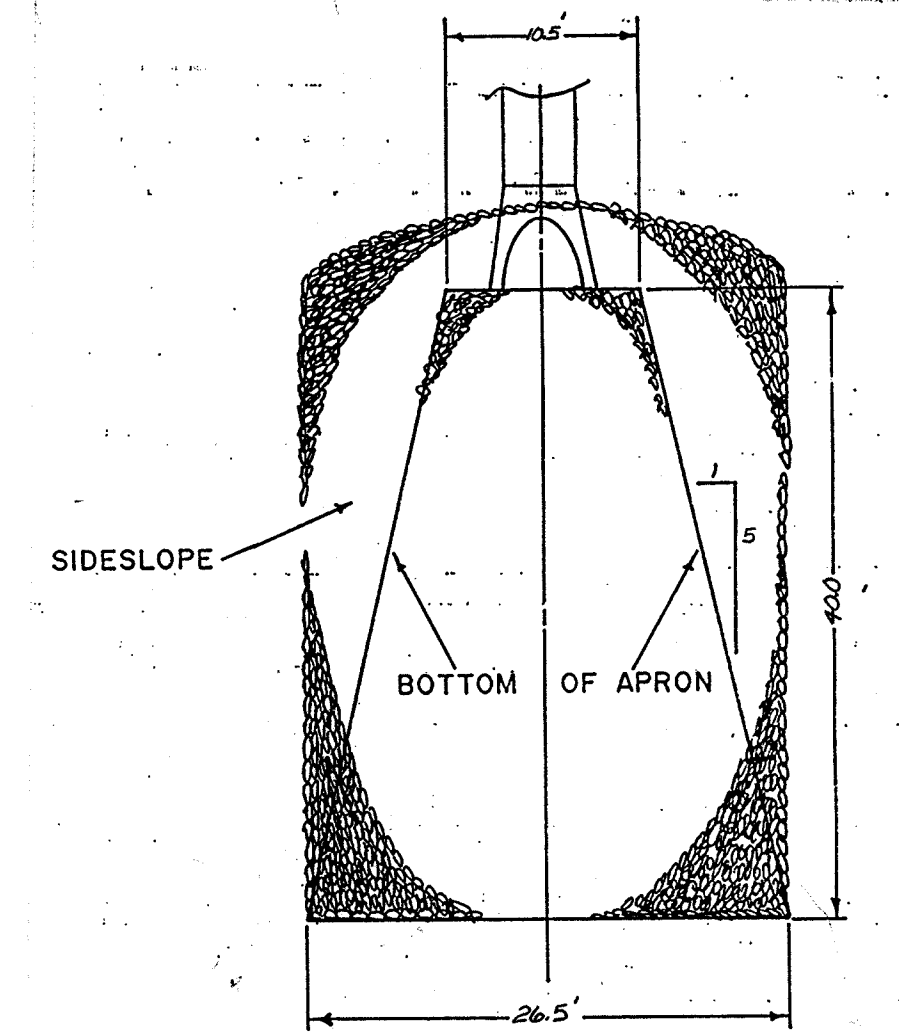
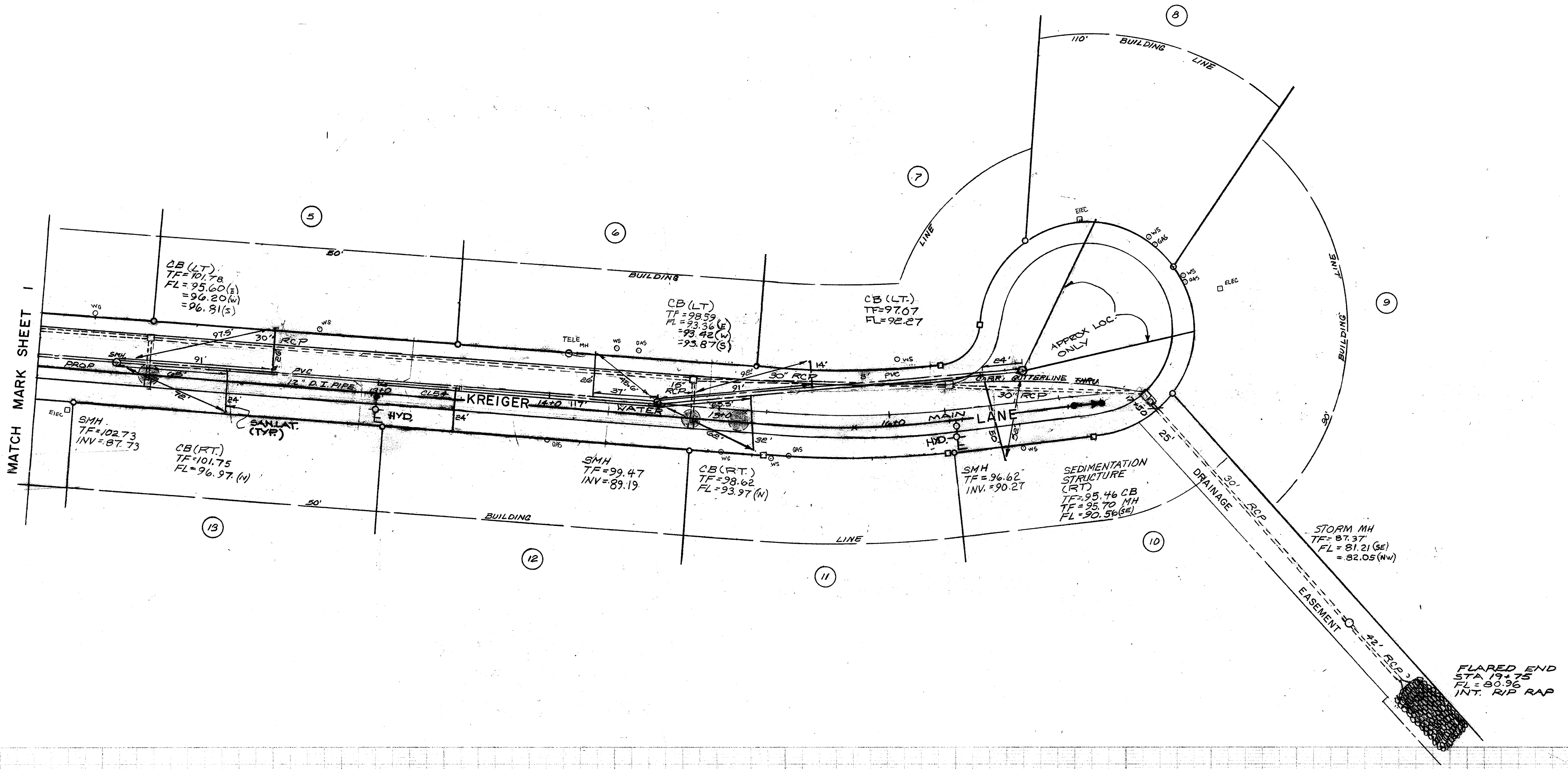
DATE APPLICATION RECEIVED COMMUNITY DEVELOPMENT DIRECTOR

DATE PUBLIC IMPROVEMENTS APPROVED DIRECTOR OF PUBLIC WORKS

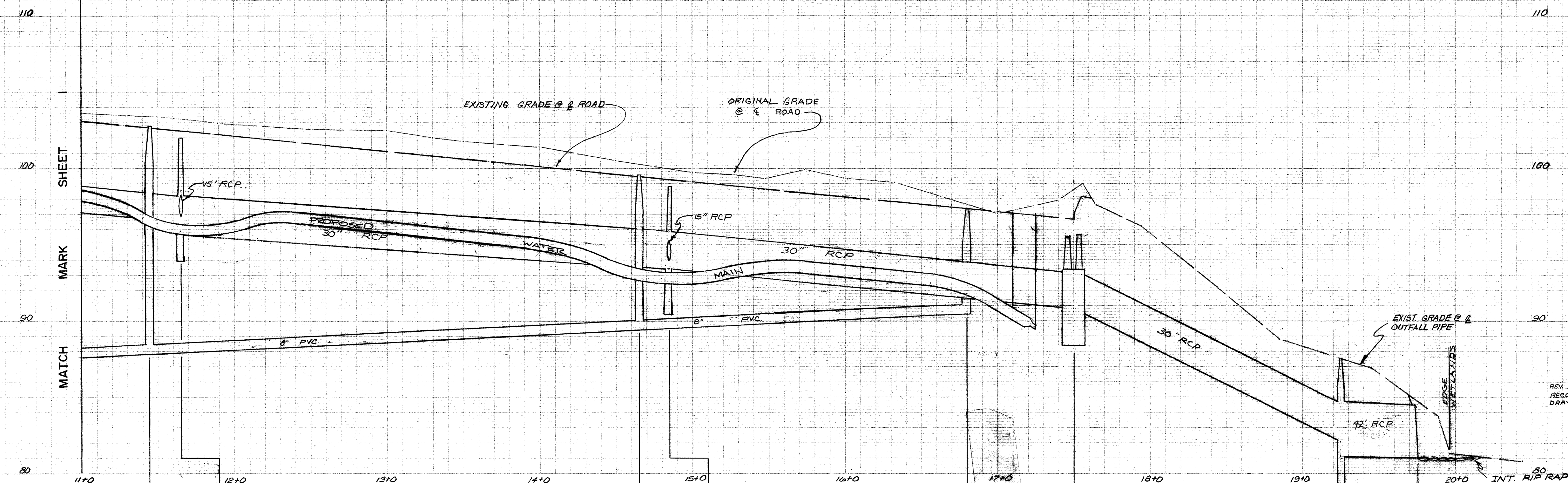
DATE SUBDIVISION APPROVED PLAN AND ZONING COMMISSION CHAIRMAN

DATE SUBDIVISION EXPIRES PLAN AND ZONING COMMISSION SECRETARY

FILE NO.



SCALE:
 PLAN: 1" = 40'
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 1" = 4' VERT.



"RECORD DRAWING"
 ROAD DRAINAGE
 SANITARY & WATER
 PLAN & PROFILE
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 OAK STREET INDUSTRIAL SUBDIVISION
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 GLASTONBURY, CONN.

DATE: 12-18-86 SCALE SHOWN MAP NO: 94-84-ID
 SHEET 2 OF 2 SHEETS